







WORK ITEMS

- (1) Substructure widening
- (2) Bearing replacement
- *3 Pile cap retrofit*
- (4) Crossbeam enlargement
- (5) Substructure replacement
- (6) Superstructure widening
- (7) Bridge rail retrofit
- (8) Column enlargement
- (9) Micropiles

LEGEND:



Ground improvements



Rotation: 189.2809° Scale: 1"=100





nt Column
ev. (ft.)
16.0
-11.0
-11.0
-5.0



PLAN Scale: 1"=40'-0"

GENERAL NOTES:

- 1. Elevations are based on North American Vertical Datum (1988).
- 2. See Geotechnical Data Report for data obtained through cone penetrometer testing and OYO suspension logging.
- 3. 1' Contour Interval
- 4. Approximate location of boring 23 is shown for information only. The geotechnical report (Dames & Moore, 1966) is included in the Geotechnical Engineering Report for the Abernethy Bridge, provided as a reference document.



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Rotation: 91.8036° Scale: 1"=40'



Sandy silty GRAVEL and Silty GRAVEL with some sand, with cobbles; GM; Brown, light brown, dark brown, and gray: Nonplastic to low plasticity fines; Moist to wet;

brown, gray, dark brown, dark gray, blue-gray, dark

medium plasticity; Moist to wet; Medium stiff to hard; Some zones of relict rock texture; Some zones of fill SILT to SILT with some sand and SILT with some sand with trace gravel; ML; Gray, dark brown, and dark gray; Nonplastic to low plasticity; Moist to wet; Loose to very dense and medium stiff to hard; Some zones of fill

with trace gravel; SM, SM/ML; Brown, light brown, dark

Sandy SILT; ML; Gray, dark gray, and brown; Nonplastic to low plasticity; Moist to wet; Soft and medium dense

Sandy GRAVEL with some silt; GP-GM; Brown and gray to tan, brown, gray, dark brown, and black; Nonplastic Sandy GRAVEL with trace silt; GP; Dark brown to black; Nonplastic fines; Wet; Very dense

Organic clavev SILT with trace sand: OH: Dark grav: High plasticity; Wet; Medium stiff to stiff

Clayey SILT; MH; Dark gray and brown; Medium to high plasticity; Moist to wet; Very stiff

CLAY to CLAY with trace sand; CH; Gray, dark gray, brown, tan, and dark green; Medium to high plasticity; Moist; Very stiff to hard; Some zones of relict rock texture

Clayey SAND with some gravel; SC; Orange-brown, dark gray, brown, and green-gray; Low to medium plasticity fines; Moist to wet; Very dense; Relict rock texture

WEATHERED BASALT; Brown-gray to gray and orange-brown; Predominantly decomposed; (R0-R1); Very close to close jointed

BASALT; Gray and brown; Predominantly decomposed to fresh; (R1-R5); Very close to wide jointed

1. Elevations are based on North American Vertical Datum (1988).

2. Boring TB19786-029 was sampled with a hammer efficiency of 88% and borings TB19876-030A and TB19876-030B

See Geotechnical Data Report for data obtained through cone penetrometer testing and OYO suspension logging. 4. Approximate location of boring 23 is shown for information only. The geotechnical report (Dames & Moore, 1966) is included in the Geotechnical Engineering Report for the Abernethy Bridge, provided as a reference document. Geotechnical data shown on this drawing are a consolidation of information and/or revision in terminology from the drill logs. More detailed subsurface data is available on the drill logs in the geotechnical report, which is available

6. Refer to the ODOT Soil and Rock Classification Manual (1987) for a description of the terms used on this sheet. 7. Borings were drilled using mud rotary and coring drilling techniques, which make it difficult to discern depth to groundwater during drilling, if it is encountered, due to the use of drilling fluid in the boreholes. 8. BOULDER ADVISORY: Boulders and cobbles were encountered during drilling for this and other nearby features and are noted on the drill logs. Boulders and cobbles may be encountered throughout the project area.

- PI	GEOTECHNICAL AND ENVIRONM GEOTECHNICAL AND ENVIRONM 3990 CAllins Way, S Lake Oswego, Oreg (503) 210-4750 FAX: (20 www.shannonwiso	LSON, INC. INTAL CONSULTANTS Suite 100 on 97035 6) 695-6777 n.com	DEPARTACIÓN CONCERCIÓN CONCE
F CUMPT	WILLAMETTE R & HWYS 1E & 3,	HWY 64 (GEO ABE	RNETHY)
NUMPER TO FREE	I-205: I-5 - OR213, EAST PORTLANE CLACKAMAS (PHASE 1 SEC. FREEWAY COUNTY	
A. Cally.	Designer: James Walters Re	eviewer: Risheng "Park" Pi	ао
	Drafter: Aimee Holmes Ch	necker: Cody Sorensen	
			SHEET NO.
	FIER I GEOTECHNICAL	DATA-2	JBC04

Rotation: 0° Scale: 1"=40'



PLAN Scale: 1"=40'-0"

GENERAL NOTES:

- 1. Elevations are based on North American Vertical Datum (1988).
- 2. See Geotechnical Data Report for data obtained through cone penetrometer testing and OYO suspension logging.
- 3. 1' Contour Interval
- 4. Approximate location of boring 22 is shown for information only. The geotechnical report (Dames & Moore, 1966) is included in the Geotechnical Engineering Report for the Abernethy Bridge, provided as a reference document.



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LEGEND



= Boring location (See Note 4) Cone Penetrometer Test Location (See Note 2)

Rotation: 91.8036° Scale: 1"=40'



Rotation: 0° Scale: 1"=40

PLAN Scale: 1"=40'-0"

GENERAL NOTES:

- 1. Elevations are based on North American Vertical Datum (1988).
- 2. See Geotechnical Data Report for data obtained through cone penetrometer testing and OYO suspension logging.
- 3. 1' Contour Interval
- 4. Approximate location of borings TB-501 and 4 are shown for information only. The geotechnical reports (ODOT, 1999, and Dames & Moore, 1966) are included in the Geotechnical Engineering Report for the Abernethy Bridge, provided as a reference document.

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LEGEND

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= Boring location (See Note 4) Cone Penetrometer Test Location (See Note 2)

FINAL ELECTRONIC DOCUMEN AVAILABLE UPON REQUEST

Rotation: 111.5447° Scale: 1"=40'

TB19786-004

Hardness

R3

R4

R4

R4

R4

R3-R4

R.

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Q.D.	qu (psi)
32	7,521
98	
98	
00	
00	
94	

Clayey SILT to Clayey SILT with trace sand; MH; Gray, brown, orange-brown, orange-yellow, red, dark brown, and white; Medium to high plasticity; Moist to wet; Stiff to very hard; Some zones of relict rock texture

Gravelly SILT with some sand, with cobbles; ML; Gray; Low plasticity; Wet; Very stiff

Silty CLAY with trace sand or trace gravel; CL; Gray, gray-brown, and orange-brown; Medium plasticity; Moist to wet; Medium stiff to very stiff

CLAY to CLAY with trace sand; CH; Orange-brown, gray, and yellow; Medium to high plasticity; Moist; Very stiff to hard; Some zones of relict rock texture

BASALT BOULDER; Dark gray; Fresh; (R4–R5)

Clayey SAND with trace gravel; SC; Orange, brown, red, yellow and black; Low plasticity fines; Moist; Very dense

WEATHERED BASALT; Gray-brown; (R1-R2); Moderately weathered; Based on drill action and cuttings

BASALT; Dark gray, gray, red, red-gray, and gray-red; Fresh to moderately weathered; (R3-R4); Very close to wide iointed

TB19786-033 % Rec. Hardness R.Q.D. qu (psi) Core C-1 100 R4-R5 42 ____ С-2 80 R0-R5 0 ____ 12,403 С-3 100 R3-R4 80 R3-R4 C-4 90 90 ____ C-5 100 R3-R4 99 7,946 С-6 100 R3-R4 88 ___ C-7 R4 97 97 ___ С-8 83 R4 83 ____ С-9 100 R4 100 ____ C-10 97 R4 82 ___ C-11 100 R4 90 ___

Boring TB19786-004 was sampled with a hammer efficiency of 88% and borings TB19786-033 and TB19786-081 were sampled with a hammer efficiency of 87%. The hammer efficiency for boring TB19786-240 is unavailable.

3. Approximate location of borings 4 and TB-501 are shown for information only. The geotechnical reports (ODOT, 1999, and Dames & Moore, 1966) are included in the Geotechnical Engineering Report for the Abernethy Bridge, provided as a reference document. 4. See Geotechnical Data Report for data obtained through cone penetrometer testing and OYO suspension logging.

Geotechnical data shown on this drawing are a consolidation of information and/or revision in terminology from the drill logs. More detailed subsurface data is available on the drill logs in the geotechnical report, which is available from the Engineer.

Refer to the ODOT Soil and Rock Classification Manual (1987) for a description of the terms used on this sheet. Borings were drilled using mud rotary, rotosonic, and coring drilling techniques, which make it difficult to discern depth to

BOULDER ADVISORY: Boulders and cobbles were encountered during drilling for this and other nearby features and are noted on the drill logs. Boulders and cobbles may be encountered throughout the project area.

FINAL ELECTRONIC DOCUMEN AVAILABLE UPON REQUEST

Rotation: 0° Scale: 1"=40

GENERAL NOTES:

- 1. Elevations are based on North American Vertical Datum (1988).
- 2. See Geotechnical Data Report for data obtained through cone penetrometer testing and OYO suspension logging.
- 3. 1' Contour Interval
- 4. Approximate location of borings TB-507, 1A, and 1 are shown for information only. The geotechnical reports (ODOT, 1999, and Dames & Moore, 1966) are included in the Geotechnical Engineering Report for the Abernethy Bridge, provided as a reference document.

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LEGEND

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= Boring location (See Note 4) Cone Penetrometer Test Location (See Note 2)

Rotation: 111.5447° Scale: 1"=40'

Hardness

R4

R2-R4

R3

R2-R3

R0-R3

R3

R4

R.Q.D.

53

16

0

0

17

65

100

Clayey SILT to Clayey SILT with trace sand; MH; Gray and o range-brown to gray-green; Medium plasticity; Moist; Stiff to very stiff; Some zones of relict rock texture

CLAY with trace sand; CH; Red, orange, yellow, and gray; Medium to high plasticity; Moist; Very hard; Relict rock texture

VOLCANIC/BASALT BRECCIA; Red, dark red, dark gray, green, orange, and brown; Moderately weathered to predominantly decomposed; (R0-R3); Very close to close *jointed; Gravel- to cobble-sized clasts in a fine-grained* matrix; Flow contact

BASALT; Dark gray, red, gray, black, orange, brown, and green; Fresh to highly weathered; (R2–R4); Very close to moderately close jointed

qu (psi)	Core	% Rec.	Hardness	R.Q.D.	qu (psi)
15,862	C-1	100	R4-R5	86	
	С-2	100	R0-R5	96	
	С-3	98	R3-R4	92	8,826
	C-4	100	R3-R4	100	
	C-5	100	R3-R4	70	
	С-6	100	R3-R4	100	
	C-7	100	R4	100	
	С-8	100	R4	98	
	С-9	97	R4	67	
	C-10	98	R4	64	

TB19786-005

1. Elevations are based on North American Vertical Datum (1988).

2. Boring TB19786-005 was sampled with a hammer efficiency of 88% and boring TB19786-034 was sampled with a

3. Approximate location of borings TB-507, 1, and 1A are shown for information only. The geotechnical reports (ODOT, 1999, and Dames & Moore, 1966) are included in the Geotechnical Engineering Report for the Abernethy Bridge,

4. See Geotechnical Data Report for data obtained through OYO suspension logging. Geotechnical data shown on this drawing are a consolidation of information and/or revision in terminology from the drill logs. More detailed subsurface data is available on the drill logs in the geotechnical report, which is available

6. Refer to the ODOT Soil and Rock Classification Manual (1987) for a description of the terms used on this sheet. 7. Artesian groundwater flow was encountered at depth during drilling. See Geotechnical Data Report for information. 8. BOULDER ADVISORY: Boulders and cobbles were encountered during drilling for this and other nearby features and are noted on the drill logs. Boulders and cobbles may be encountered throughout the project area.

OR ⁴	SHANNON & WILSON, INC. GETECHNICAL AND ENVIRONMENTAL CONSULTANTS 3990 Collins Way, Suite 100 Lake Oswego, Oregon 97035 (503) 210-4750 FAX: (206) 695-6777 www.shannonwison.com	OL DEPARTALE
ADVANCE COUNTRY	WILLAMETTE R & HWYS 1E & 3, HWY 64 (GEO ABE I-205: I-5 - OR213, PHASE 1 SEC. EAST PORTLAND FREEWAY CLACKAMAS COUNTY	RNETHY)
Ar cillo.	Designer: James Walters Reviewer: Risheng "Park" Pi	ao
-	Drafter: Aimee Holmes Checker: Cody Sorensen	
		SHEET NO.
	PIER 4 GEOTECHNICAL DATA - 2	JBC10

Rotation: 0° Scale: 1"=40'

PLAN Scale: 1"=40'-0"

- 1. Elevations are based on North American Vertical Datum (1988).
- 2. See Geotechnical Data Report for data obtained through cone penetrometer testing and OYO suspension logging.
- 3. 1' Contour Interval
- 4. Approximate locations of borings 2A and 2 are shown for information only. The geotechnical report (Dames & Moore, 1966) is included in the Geotechnical Engineering Report for the Abernethy Bridge, provided as a reference document.

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LEGEND

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= Boring location (See Note 4) Cone Penetrometer Test Location (See Note 2)

FINAL ELECTRONIC DOCUMEN AVAILABLE UPON REQUEST

Rotation: 111.5447° Scale: 1"=40'

drilling action	Silty SAND and Silty SAND with trace gravel; SM; Gray, brown-orange, orange-brown, brown, and orange; Nonplastic to low plasticity fines; Moist to wet; Medium dance to vary dance. Some zones of relist rock toyture
nd GRAVEL with ay, dark gray, Wet; Loose to	Sandy CLAY; CH; Orange-brown, black, gray, pink, red, and green; Medium plasticity; Moist; Hard to very hard; Relict rock texture
rown to gray; e	Silty CLAY with some sand with no to trace gravel; CL; Gray, orange, brown, orange-brown, yellow-green, and yellow; Low to medium plasticity; Moist to wet; Hard to very hard; Relict rock texture
<i>-orange, and et; Loose/m rock texture</i>	<i>Clayey SILT to Sandy clayey SILT; MH; Brown-orange and gray; Medium plasticity; Moist to wet; Very stiff to hard; Some zones of relict rock texture</i>
fines; Wet;	Clayey SAND; SC; Orange and brown; High plasticity fines; Based on drill action and cuttings
icity; Wet;	WEATHERED BASALT; Gray-brown and orange-brown; Predominantly decomposed to moderately weathered; (R0-R2); Remolds to Silty GRAVEL with some sand (GM)
ange, to high	BASALT; Dark gray, dark gray-red, brown, yellow, and green-gray; Fresh to predominantly decomposed; (R0-R5); Very close to moderately close jointed

qu (psi)	

TB19786-035

qu (psi)	Core	% Rec.	Hardness	R.Q.D.	qu (psi)
	C-1	71	R0-R3	17	
	С-2	100	R2	50	1,249
	С-3	97	R2-R4	0	
	C-4	100	R4	71	9,453
	C-5	100	R4	78	
	С-6	100	R4-R5	93	
	C-7	100	R4-R5	85	
	С-8	100	R4-R5	94	
9,952	С-9	75	R4-R5	42	

2. Boring TB19786-006 was sampled with a hammer efficiency of 88% and boring TB19786-035 was sampled with a

3. Approximate locations of borings 2 and 2A are shown for information only. The geotechnical report (Dames & Moore, 1966) is included in the Geotechnical Engineering Report for the Abernethy Bridge, provided as a reference

4. See Geotechnical Data Report for data obtained through OYO suspension logging. Geotechnical data shown on this drawing are a consolidation of information and/or revision in terminology from the drill logs. More detailed subsurface data is available on the drill logs in the geotechnical report, which is available

6. Refer to the ODOT Soil and Rock Classification Manual (1987) for a description of the terms used on this sheet. 7. Artesian groundwater flow was encountered at depth during drilling. See Geotechnical Data Report for information. 8. BOULDER ADVISORY: Boulders and cobbles were encountered during drilling for this and other nearby features and are noted on the drill logs. Boulders and cobbles may be encountered throughout the project area.

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CU MBE	WILLAMETTE R & HWYS 1E & 3, HWY 64 (GEO ABERNETHY)			
TUCH	I-205: I-5 - OR213, PHASE 1 SEC. EAST PORTLAND FREEWAY CLACKAMAS COUNTY			
	Designer: James Walters Reviewer: Risheng "Park" Piao			
	Drafter: Aimee Holmes Checker: Cody Sorensen			
		SHEET NO.		
	FIER 5 GEOTECHNICAL DATA - 2	JBC12		

Rotation: 0° Scale: 1"=40'

Rotation: 111.5447° Scale: 1"=40'

TB19786-037

R4

R4-R5

______ R4-R5

R4-R5

R4-R5

R3-R5

100

100

98

95

100

96

STRUCTURE NO

09403

BDS DWG NO

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CALC. BOOK

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COUNTY

Clackamas

DATE

05/21

P.: 9.03

Hardness

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to CLAY with trace sand; CH; Orange-brown, yellow, red, brown-red, orange, black, and dark red-brown; um to high plasticity; Moist; Very stiff to very hard; rock texture

y SILT with some sand, Clayey SILT to CLAY, and SILT to y SILT; MH, MH/CH, ML/MH; Gray, orange-brown, brown to olive-gray, and black; Low to medium city; Moist to wet; Stiff to very hard; Some zones of basalt texture

CLAY with trace to some sand; CL; Gray-brown, prown, orange, gray, white, green, dark red-brown, v-brown, yellow, orange-brown, brown, and black; Low edium plasticity; Moist; Very stiff to very hard; Relict exture

HERED BASALT; Gray-brown, orange-brown, yellow, and red; Decomposed to moderately weathered; 3); Very close to moderately close jointed; Some zones valtered to very stiff/hard CLAY (CH)

MPOSED BRECCIA and BASALT BRECCIA; Brown, red, gray, red-gray, orange, and dark green-brown, nposed to slightly weathered; (R0-R3); Very close to jointed; Gravel- to cobble-sized fragments in a grained matrix; Some zones highly altered to very hard CLAY (CH)

T; Dark gray, gray, orange-brown, brown, brown-gray, prange; Fresh to moderately weathered; (R2–R5); Very to wide iointed

77

93

98

95

100

67

TB19786-036

Core	% Rec.	Hardness	R.Q.D.	qu (psi)
C-1	93	R3	8	
С-2	100	R3-R4	7	
С-3	100	R3	0	
C-4	100	R3	20	
C-5	98	R2-R3	98	
С-6	100	R3-R4	58	
C-7	100	R4	55	
С-8	100	R4	73	
С-9	97	R4	71	

1. Elevations are based on North American Vertical Datum (1988).

2. Borings were sampled with a hammer efficiency of 87%.

Approximate location of boring 3 is shown for information only. The geotechnical report (Dames & Moore, 1966) is included in the Geotechnical Engineering Report for the Abernethy Bridge, provided as a reference document. 4. See Geotechnical Data Report for data obtained through OYO suspension logging.

5. Geotechnical data shown on this drawing are a consolidation of information and/or revision in terminology from the drill logs. More detailed subsurface data is available on the drill logs in the geotechnical report, which is available

6. Refer to the ODOT Soil and Rock Classification Manual (1987) for a description of the terms used on this sheet. Artesian groundwater flow was encountered at depth during drilling. See Geotechnical Data Report for information. 8. BOULDER ADVISORY: Boulders and cobbles were encountered during drilling for this and other nearby features and are noted on the drill logs. Boulders and cobbles may be encountered throughout the project area.

PLAN Scale: 1"=40'-0"

GENERAL NOTES:

- 1. Elevations are based on North American Vertical Datum (1988).
- 2. See Geotechnical Data Report for data obtained through cone penetrometer testing and OYO suspension logging.
- 3. 1' Contour Interval
- 4. Approximate locations of borings 5, 6, TB-502A, and TB-502B are shown for information only. The geotechnical reports (ODOT, 1999, and Dames & Moore, 1966) are included in the Geotechnical Engineering Report for the Abernethy Bridge, provided as a reference document.

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LEGEND

- = Boring location (See Note 4)
- Cone Penetrometer Test Location = (See Note 2)

Rotation: 111.5447° Scale: 1"=40'

TB19786-038

Hardness

R4-R5

R2-R3

R0-R3

R0-R1

R0-R1

R0-R1

R0-R1

R0-R1

R1

R2-R3

R3

R3

R3

R3-R4

R4

R4

R3-R4

R3-R4

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R.Q.D. qu (psi)

12

0

0

0

0

0

0

0

0

18

22

20

0

38

58

70

50

74

20,108

105

% Rec.

80

32

26

72

40

88

44

32

52

94

68

86

40

100

96

100

100

100

CLAY with trace sand; CH; Dark red-brown; Medium plasticity; Moist; Hard; Relict rock texture

Silty CLAY with trace to some sand; CL; Red-brown, yellow, dark red-brown, yellow-brown, black-orange, and gray-green; Low to medium plasticity; Moist; Very stiff to hard; Relict rock texture

SILT to Clayey SILT; ML/MH; Olive-brown to olive-gray; Low to medium plasticity; Moist; Very hard; Relict rock texture

Sandy silty CLAY with trace gravel; CL; Gray, green-gray, and red-brown; Low plasticity; Moist; Hard; Relict rock texture

FAULT BRECCIA, DECOMPOSED BRECCIA, and BASALT BRECCIA; Gray, light gray, pink, orange, tan, black, brown, red, dark green-brown, red-gray, orange-brown, dark brown, yellow, and white; Decomposed to slightly weathered; (R0-R3); Very close to moderately close jointed; Gravel- to boulder-sized rock fragments in a silt to coarse sand *matrix; Occasional slightly weathered (R3–R4) basalt boulders*

WEATHERED BASALT; Gray-brown, orange-brown, brown, brown-gray, gray, red, yellow, and yellow-brown; Decomposed to moderately weathered; (R0-R3); Very close to moderately close jointed; Some zones highly altered to CLAY (CH)

BASALT; Gray, dark gray, and brown-gray; Fresh to moderately weathered; (R2-R5); Very close to moderately close jointed

GENERAL NOTES:

- 1. Elevations are based on North American Vertical Datum (1988).
- 2. Borings TB19786-014 and TB19786-038 were sampled with a hammer efficiency of 88% and boring TB19876-132 was sampled with a hammer efficiency of 90%.
- 3. Approximate locations of borings 5, 6, TB-502A, and TB-502B are shown for information only. The geotechnical reports (ODOT, 1999, and Dames & Moore, 1966) are included in the Geotechnical Engineering Report for the Abernethy Bridge, provided as a reference document.
- *4. See Geotechnical Data Report for data obtained through cone* penetrometer testing and OYO suspension logging.
- 5. Geotechnical data shown on this drawing are a consolidation of information and/or revision in terminology from the drill logs. More detailed subsurface data is available on the drill logs in the geotechnical report, which is available from the Engineer.
- 6. Refer to the ODOT Soil and Rock Classification Manual (1987) for a description of the terms used on this sheet.
- 7. Borings were drilled using mud rotary and coring drilling techniques, which make it difficult to discern depth to groundwater during drilling, if it is encountered, due to the use of drilling fluid in the boreholes. Artesian groundwater flow was encountered at depth during drilling in boring TB19786-038. See Geotechnical Data Report for information.
- 8. BOULDER ADVISORY: Boulders and cobbles were encountered during drilling for this and other nearby features and are noted on the drill logs. Boulders and cobbles may be encountered throughout the project area.

FINAL ELECTRONIC DOCUMEN AVAILABLE UPON REQUEST

Rotation: 0°

Scale: 1"=40

PLAN Scale: 1"=40'-0"

GENERAL NOTES:

- 1. Elevations are based on North American Vertical Datum (1988).
- 2. See Geotechnical Data Report for data obtained through cone penetrometer testing and OYO suspension logging.
- 3. 1' Contour Interval
- 4. Approximate location of boring 7 is shown for information only. The geotechnical report (Dames & Moore, 1966) is included in the Geotechnical Engineering Report for the Abernethy Bridge, provided as a reference document.

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LEGEND

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= Boring location (See Note 4) Cone Penetrometer Test Location (See Note 2)

Rotation: 111.5447° Scale: 1"=40'

51

84

84

26

46

58

30

30

60

100

92

32

31

86

6.951

8,311

qu (psi)

5,493

8,033

12,723

STRUCTURE NO

09403

BDS DWG NO

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CALC. BOOK

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COUNTY

Clackamas

DATE

05/21

HWY∙ 064

P • 9.03

CLAY to CLAY with some sand and trace gravel and CLAY to Clayey SILT with some sand and trace gravel; CH, CH/MH; Red, brown, gray, yellow, blue, red-brown, orange, red-orange mottled, white mottled, and multicolored; Medium to high plasticity; Damp to wet; Medium stiff to hard; Relict rock texture

Clayey SILT with trace to some sand, Clayey SILT with sand and gravel, and Sandy clayey SILT with trace gravel; MH; Red, red-orange, brown-red, brown, orange, yellow, gray-orange, gray, dark brown, orange-brown, red-brown, yellow-brown, green, blue-gray, dark red-brown, and gray-brown; Low to high plasticity; Moist; Stiff to hard; Relict rock texture

Silty CLAY with trace sand and gravel; CL; Red-orange mottled; Low to medium plasticity; Moist to wet; Very stiff; Relict rock texture

Gravelly silty CLAY; CL; Gray mottled orange-brown; Low to medium plasticity; Moist; Hard; Relict rock texture

WEATHERED BASALT and BASALT; Gray, brown, orange, dark gray-brown, dark brown, orange-brown, vellow-brown, and brown-grav; Decomposed to moderately weathered; (R0-R3); Very close to moderately close jointed; Some zones remold to Sandy SILT with some gravel (ML) and Clayey SAND with trace gravel (SC)

BASALT; Gray, dark gray, brown-gray, red-brown, black, red-gray, and blue-gray; Fresh to moderately weathered; (R3-R4); Very close to moderately close jointed

1619760-250					
Core	% Rec.	Hardness	R.Q.D.	qu (psi)	
C-1	100	R3-R4	70	20,108	
С-2	100	R3-R4	96		
С-3	90	R3-R4	54		
C-4	68	R3-R4	0	105	
C-5	100	R4	73		

TR10706 226

GENERAL NOTES:

1. Elevations are based on North American Vertical Datum (1988).

2. Borings TB19786-008 and TB19786-041 were sampled with a hammer efficiency of 88%, borings TB19786-235 and TB19786-236 were sampled with a hammer efficiency of 86%, and boring TB19876-040 was sampled with a hammer efficiency of 90%.

3. Approximate location of boring 7 is shown for information only. The geotechnical report (Dames & Moore, 1966) is included in the Geotechnical Engineering Report for the Abernethy Bridge, provided as a reference document.

See Geotechnical Data Report for data obtained through cone penetrometer testing and OYO suspension logging.

Geotechnical data shown on this drawing are a consolidation of information and/or revision in terminology from the drill logs. More detailed subsurface data is available on the drill logs in the geotechnical report, which is available from the Engineer.

6. Refer to the ODOT Soil and Rock Classification Manual (1987) for a description of the terms used on this sheet.

Borings were drilled using mud rotary and coring drilling techniques, which make it difficult to discern depth to groundwater during drilling, if it is encountered, due to the use of drilling fluid in the boreholes.

8. BOULDER ADVISORY: Boulders and cobbles were encountered during drilling for this and other nearby features and are noted on the drill logs. Boulders and cobbles may be encountered throughout the project area.

- PI	SHANNON & WIL GEOTECHNICAL AND ENVIRONME 3990 Collins Way, S Lake Oswego, Orego (503) 210-4750 FAX: (206 www.shannon/Wsor	SON, INC. NTAL CONSULTANTS uite 100 nn 97035 si) 695-6777 Lcom	ST DEPARTMENT ST DEPARTMENT OF THE ST DEPARTMENT OF	
NUMPER TO CHINE	WILLAMETTE R & HWYS 1E & 3, HWY 64 (GEO ABERNETHY) I-205: I-5 - OR213, PHASE 1 SEC. EAST PORTLAND FREEWAY CLACKAMAS COUNTY			
the caller	Designer: James Walters Re	Reviewer: Risheng "Park" Piao		
-	Drafter: Aimee Holmes Ch	ecker: Cody Sorensen		
			SHEET NO.	
	FIER 8 GEOTECHNICAL	DATA - Z	JBC18	

PLAN Scale: 1"=40'-0"

GENERAL NOTES:

- 1. Elevations are based on North American Vertical Datum (1988).
- 2. See Geotechnical Data Report for data obtained through cone penetrometer testing and OYO suspension logging.
- 3. 1' Contour Interval
- 4. Approximate locations of borings 8, 19, and TB-503 are shown for information only. The geotechnical reports (Dames & Moore, 1966, and ODOT, 1999) are included in the Geotechnical Engineering Report for the Abernethy Bridge, provided as a reference document.

DRAFT - NOT FOR CONSTRUCTION - SUBJECT TO CHANGE

LEGEND

=

= Boring location (See Note 4) Cone Penetrometer Test Location (See Note 2)

Rotation: 111.5447° Scale: 1"=40'

PLAN Scale: 1"=40'-0"

GENERAL NOTES:

- 1. Elevations are based on North American Vertical Datum (1988).
- 2. See Geotechnical Data Report for data obtained through cone penetrometer testing and OYO suspension logging.
- 3. 1' Contour Interval
- 4. Approximate locations of borings 9, 9A, 17, 18, and TB-504 are shown for information only. The geotechnical reports (Dames & Moore, 1966, and ODOT, 1999) are included in the Geotechnical Engineering Report for the Abernethy Bridge, provided as a reference document.

DRAFT - NOT FOR CONSTRUCTION - SUBJECT TO CHANGE

LEGEND

- = Boring location (See Note 4)
- Cone Penetrometer Test Location = (See Note 2)

Rotation: 111.5447° Scale: 1"=40'

PLAN Scale: 1"=40'-0"

GENERAL NOTES:

- 1. Elevations are based on North American Vertical Datum (1988).
- 2. See Geotechnical Data Report for data obtained through cone penetrometer testing and OYO suspension logging.
- 3. 1' Contour Interval
- 4. Approximate locations of borings 10, 10A, 20, 21, and TB-505 are shown for information only. The geotechnical reports (Dames & Moore, 1966, and ODOT, 1999) are included in the Geotechnical Engineering Report for the Abernethy Bridge, provided as a reference document.

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LEGEND

- = Boring location (See Note 4)
- Cone Penetrometer Test Location = (See Note 2)

Rotation: 111.5447° Scale: 1"=40'

GENERAL NOTES:

- 1. Elevations are based on North American Vertical Datum (1988).
- 2. See Geotechnical Data Report for data obtained through cone penetrometer testing and OYO suspension logging.
- 3. 1' Contour Interval
- 4. Approximate locations of borings 11, 15, 16, 21, 25, and TB-506 are shown for information only. The geotechnical reports (Dames & Moore, 1966, and ODOT, 1999) are included in the Geotechnical Engineering Report for the Abernethy Bridge, provided as a reference document.

DRAFT - NOT FOR CONSTRUCTION - SUBJECT TO CHANGE

LEGEND

- = Boring location (See Note 4)
- Cone Penetrometer Test Location = (See Note 2)

Rotation: 111.5447° Scale: 1"=40'

Rotation: 0° Scale: 1"=40

PLAN Scale: 1"=40'-0"

GENERAL NOTES:

- 1. Elevations are based on North American Vertical Datum (1988).
- 2. See Geotechnical Data Report for data obtained through cone penetrometer testing and OYO suspension logging.
- 3. 1' Contour Interval
- 4. Approximate locations of borings 12 and 12A are shown for information only. The geotechnical report (Dames & Moore, 1966) is included in the Geotechnical Engineering Report for the Abernethy Bridge, provided as a reference document.

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LEGEND

- = Boring location (See Note 4)
- Cone Penetrometer Test Location = (See Note 2)

Rotation: 111.5447° Scale: 1"=40'

UNIT DESCRIPTIONS

Sandy GRAVEL with some silt and GRAVEL with some silt and sand, with cobbles; GP-GM; Brown to gray; Nonplastic to low plasticity fines; Moist to wet; Medium dense to very dense; Some zones of fill

Silty CLAY with some sand and no to trace gravel; CL; Dark gray, brown mottled, and multicolored; Low to high plasticity; Moist; Stiff to very

Silty GRAVEL with some sand, with cobbles; GM; Brown and gray; Nonplastic fines; Moist; Dense to very dense; Some zones of fill

Sandy SILT; ML; Red-brown mottled; Low plasticity; Moist; Medium stiff

WEATHERED BASALT; Based on drill action and cuttings

BASALT; Gray and orange-brown; Slightly to highly weathered; (R1-R4); Very close to close jointed

FAULT BRECCIA; Red, gray, green, white mottled, and multicolored; Moderately weathered to predominantly decomposed; (R0-R4); Very close to moderately close jointed; Sand- to boulder-sized fragments in a fine-grained matrix

1. Elevations are based on North American Vertical Datum (1988).

2. Borings were sampled with a hammer efficiency of 90%.

3. Approximate locations of borings 12 and 12A are shown for information only. The geotechnical report (Dames & Moore, 1966) is included in the Geotechnical Engineering Report for the Abernethy Bridge, provided as a reference document.

4. See Geotechnical Data Report for data obtained through cone penetrometer testing and

5. Geotechnical data shown on this drawing are a consolidation of information and/or revision in terminology from the drill logs. More detailed subsurface data is available on the drill logs in the geotechnical report, which is available from the Engineer. 6. Refer to the ODOT Soil and Rock Classification Manual (1987) for a description of the

7. Borings were drilled using mud rotary and coring drilling techniques, which make it difficult to discern depth to groundwater during drilling, if it is encountered, due to the use of drilling fluid in the boreholes.

8. BOULDER ADVISORY: Boulders and cobbles were encountered during drilling for this and other nearby features and are noted on the drill logs. Boulders and cobbles may be encountered throughout the project area.

THATE FORMER	WILLAMETTE R & HWYS 1E & 3, HWY 64 (GEO ABERNETHY) H-205: I-5 - OR213, PHASE 1 SEC. EAST PORTLAND FREEWAY CLACKAMAS COUNTY		
A. Silp.	Designer: James Walters	Reviewer: Risheng "Park" Piao	
			SHEET NO.
	PIER 13 GEUTECH	NICAL DATA - 2	JBC28

Rotation: 0° Scale: 1"=40'

FINAL ELECTRONIC DOCUMEN AVAILABLE UPON REQUEST

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BR_K19786_HDR_ftg_16.dgn :: Default 4/25/2021 12:52:44 PM HGONSIOR

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1. Top of bedrock elevation may vary. See special provisions to accommodate variation. Tip elevation may vary to construct required socket into bedrock.

2. See Special Provisions for permanent structural casing requirements. plicing of permanent structural casing segments requires complete penetration welds. 3. Where the min. thickness of permanent structural casing is shown, it is specified to satisfy structural design requirements only. The contractor shall increase the casing thickness to provide casing of sufficient strength to resist handling, transportation, and installation stresses and the external stresses of the subsurface materials.

4. Do not splice more than 50% of reinf. bar at any one location. Stagger splices a

5. For #8 hoops, see sheet JBD30 for Welded Lap Splice Detail.

Provide all #9 or larger reinforcing steel according to ASTM Specification A706, Grade 80.

HDR ENGINEERING, INC 1050 SW 6TH AVENUE, SUITE 1800 PORTLAND, OR 97204-1134 503.423.3700 WILLAMETTE R & HWYS 1E & 3, HWY 64 (GEO ABERNETHY) I-205: I-5 - OR213, PHASE 1 SEC. EAST PORTLAND FREEWAY CLACKAMAS COUNTY Designer: Jedediah Bingle Reviewer: Jeff Olson Drafter: Jade Wang Checker: Quincy Engineering SHEET NO. PIER 4 SHAFT 4B JBD17

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1. Top of bedrock elevation may vary. See special provisions to accommodate variation. Tip elevation may vary to construct required socket into bedrock.

2. Do not splice more than 50% of reinf. bar at any one location. Stagger splices

3. For #8 hoops, see sheet JBD30 for Welded Lap Splice Detail.

Provide all #9 or larger reinforcing steel according to ASTM Specification A706, Grade 80.

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WILLAMETTE R & HWYS 1E & 3, HWY 64 (GEO ABERNETHY) Checker: Quincy Engineering SHEET NO. JBD20

FINAL ELECTRONIC DOCUMEN AVAILABLE UPON REQUEST

Rotation: 0° Scale: 3/16"=1'-0'

1. Top of bedrock elevation may vary. See special provisions to accommodate variation. Tip elevation may vary to construct required socket into bedrock.

 See Special Provisions for permanent structural casing requirements. Splicing of permanent structural casing segments requires complete penetration welds.
Where the min. thickness of permanent structural casing is shown, it is specified to satisfy structural design requirements only. The contractor shall increase the casing thickness to provide casing of sufficient strength to resist handling, transportation, and installation stresses and the external stresses of the subsurface materials.

4. Do not splice more than 50% of reinf. bar at any one location. Stagger splices a minimum of 3'-0".

5. For #8 hoops, see sheet JBD30 for Welded Lap Splice Detail. Provide all #9 or larger reinforcing steel according to ASTM Specification A706, Grade 80.

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1. Top of fault breccia elevation may vary. See Special Provisions to accommodate variation.

4. Shaft diameter above estimated top of fault breccia to be determined by contractor with

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SHEET NO. JBD27

Welding shall meet the requirements of Specification Section 00530.42(d). for weld dimensions, see table below.

SHAFT SPIRAL OPTIONS

	Weld dimensions (in.)		
	5	Ε	Length (L)
#4	1/4	1/8	4
#5	<i>₹</i> 16	³ /16	6
#6	31 ₈	³ /16	6

SHAFT HOOP OPTIONS

	Weld dimensions (in.)		
	5	E	Length (L)
#7	7_{16}	1/4	7
#8	1/2	1/4	8
#9	9 ₁₆	⁵ /16	8

Rotation: 111.7268° Scale: 3/32" = 1'-0"

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FINAL ELECTRONIC DOCUMEN AVAILABLE UPON REQUEST

Rotation: 91.8035° Scale: 1"=15'

El. 142.25

<u>-10"</u> typ.

HDR ENGINEERING, INC 1050 SW 6TH AVENUE, SUITE 1800 PORTLAND, OR 97204-1134 503.423.3700 WILLAMETTE R & HWYS 1E & 3, HWY 64 (GEO ABERNETHY) I-205: I-5 - OR213, PHASE 1 SEC. EAST PORTLAND FREEWAY CLACKAMAS COUNTY Designer: Kristopher Walke Reviewer: Jeff Olson Drafter: David Massingale Checker: Quincy Engineering SHEET NO. PIER 13 GEOMETRY PLAN AND ELEVATION JBM27

FINAL ELECTRONIC DOCUMEN AVAILABLE UPON REQUEST

Rotation: 113.9435° Scale: 1"=15'

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SHEET NO.

JBN01

SHEET NO.

JBN06